

Former Hamble Airfield, Hamble-le-Rice, Hampshire

Ground Movement Assessment for Network Rail Assets

Project reference: 331201108

On behalf of: CEMEX UK Operations Ltd



Project Ref: 331201108 | Document: RP-3501-V02 | June 2023



Document Control Sheet

Project:	Former Hamble Airfield, Hamble-le-Rice, Hampshire
Document:	Ground Movement Assessment for Network Rail Assets
Project Ref:	331201108
Document:	331201108-RP-3501-V02
Date:	June 2023

	Name	Position	Signature	Date		
Prepared by:	Martyn Higham	Senior Associate	M D Higham	18 May 2023		
Reviewed by:	Rob Foster	Associate	R Foster	18 May 2023		
Approved by:	Martyn Higham	Senior Associate	M D Higham	18 May 2023		
For and on behalf of Stantec UK Limited						

Issue	Date	Description	Prepared	Reviewed	Approved
v01a	May 2023	Issued for information	mdh	rf	mdh
v02	Jun 2023	Issued final	mdh	-	-

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1.0 Introduction

1.1 Preamble

1.1.1 Stantec UK Limited (Stantec) has been commissioned by Cemex UK Operations Ltd (the Client) to prepare an engineering appraisal of the geotechnical conditions together with an assessment of the effects of sand and gravel extraction on the existing Network Rail (NR) assets in the vicinity of the proposed works at the former Hamble Airfield, Hamble-le-Rice (the Site).

1.2 Background

- 1.2.1 It is proposed to extract sand and gravel from a site at Hamble Airfield, Hamble-le-Rice. The proposed extraction of sand and gravel is intended to be carried out in dry or wet conditions depending on the groundwater level however any areas that need to be restored using imported materials may require an attenuation layer to be installed prior to placement of the imported material. In these areas the groundwater level may have to be artificially lowered through pumping of groundwater in order to install the attenuation layer and place the imported material.
- 1.2.2 Details of the proposed development have been submitted to Hampshire County Council as part of the planning application for the scheme (Application CS/22/92277, dated 29 December 2021).
- 1.2.3 Network Rail has been given the opportunity to comment on the proposed development by the local planning authority. In their response, Network Rail stated that the applicant must demonstrate what implications localised dewatering will have on the railway line/cutting which runs parallel to the northern boundary of the site.

1.3 Scope of Work

- 1.3.1 The scope of work performed by Stantec comprises:
 - i) A review and critical appraisal of the ground investigations and other available information to determine the ground conditions in the vicinity of the Site and Network Rail assets.
 - ii) An assessment of potential ground movements associated with any localised dewatering and consequent effects on Network Rail assets in the vicinity of the Site.
- 1.3.2 This report presents an assessment of the ground conditions on the Site together with recommended values of geotechnical parameters for use in the assessment of ground movements. The report also presents details of the Network Rail assets in the vicinity of the Site and a detailed technical assessment of the effects of potential ground movements associated with any localised dewatering on those assets.

1.4 Limitations

- 1.4.1 Unless stated otherwise, information from previous studies and investigations has not been included in this report and, where referenced, the reports presenting this information should be read in conjunction with this report.
- 1.4.2 Guidance on the context of this report and any general limitations or constraints on its content and usage are given in a separate guidance note included after the text of this report.



2.0 The Site

2.1 Site Location

- 2.1.1 The Site is centred at National Grid Reference SU 477 076 about 0.3 km north of Hamble-le-Rice The location of the Site is shown on a Site Location Plan presented as Figure 1.
- 2.1.2 The Site is an irregular polygon in plan with overall plan dimensions of about 750 by 1150 m. The Site is bound to the west by the B3397 Hamble Lane with undeveloped sports pitches beyond; to the southwest and south by residential development; to the southeast by residential development along Satchell Lane; to the east by Satchell Lane with undeveloped agricultural and woodland beyond; and to the north by the Southampton to Fareham railway line. The layout of the Site is shown on a Site Layout Plan presented as Figure 2.
- 2.1.3 The Site is situated on a ridge between the valley of the River Hamble to the east and Southampton Water to the West and comprises undeveloped rough grassland partly colonised by dense scrub of the landing areas of the former Hamble North Airfield. Natural ground levels across the Site around generally between about 20.0 and 21.0 m relative to Ordnance Datum (OD) rising to about 23.0 m OD on the northern part of the Site and locally falling to about 15.0 m OD along the eastern boundary of the Site.

2.2 Network Rail Assets

- 2.2.1 The Network Rail (NR) asset in the vicinity of the Site comprises a section of the Southampton to Fareham railway line (ELR: SDP1, Mileage: 7m19ch to 7m40ch) between Hamble Station to the west and Burlesdon Station to the east. This section of the railway is aligned approximately northeast-southwest and comprises two tracks running in cutting down to about 5.0 m below the adjacent ground level. The base of the cutting is about 8 m wide, and the side slopes are formed at gradients up to about 1 vertical in 2 horizontal.
- 2.2.2 The alignment of the railway is shown on the Site Layout Plan presented as Figure 2. The elevation of the railway and natural ground level adjacent to the railway are illustrated on a Schematic Geological Sections included as Figure 3.

2.3 Ground Conditions

Geology

- 2.3.1 The 1:50,000 scale geological sheet of the area (BGS, 1987a) indicates that the solid geology in the vicinity of the Site comprises a sequence of the Selsey Sand Formation, the Marsh Farm Formation and the Earnley Sand Formation of the Bracklesahm Group. These formations dip to the southwest with the Selsey Sand Formation sub-cropping on the southwest part of the Site, the Marsh Farm Formation sub-cropping on the central part of the Site and the Earnley Sand Formation sub-cropping of the Site.
- 2.3.2 The geological memoir for the area (BGS, 1987b) indicates the Marsh Farm Formation comprises laminated clays with thin beds of sand and silt, and sand with clay beds with rapid lateral and vertical variations in the proportions of sand and clay present.
- 2.3.3 The solid geology is shown to be overlain by River Terrace Deposits. In addition, Head deposits formed by natural geomorphological processes are expected to be present overlying the River Terrace Deposits.



Ground Investigations

- 2.3.4 The ground conditions in the area of the Site were investigated by RMC Aggregates Ltd in June 1995 and by Apex Drilling Services Ltd in May 2011. In addition, water observation boreholes have been installed over the Site by D K Sykes Associates in February 2008 and by Southeastern Drilling Services Ltd in November 2018.
- 2.3.5 The 1995 ground investigation included 28 boreholes (denoted BH 01/95 to 28/95) to a maximum depth of 7.5 m, and the 2011 ground investigation 10 boreholes (denoted BH01 to 06 and BH09 to 12) to a maximum depth of 9.0 m to provide information on the available mineral resources. The 2008 ground investigation included 12 boreholes (denoted BH A/08 to L/08) to a maximum depth of 8.3 m, and the 2018 ground investigation 9 boreholes (denoted BH W01 to W06, W08 and W10 to W12) to a maximum depth of 12.5 m to allow the installation of groundwater monitoring wells.
- 2.3.6 In addition, the British Geological Survey archives contain the record of an exploratory hole (denoted BH HC in this report) about 0.35 km northwest of the Site.
- 2.3.7 The locations of the boreholes the Site are shown on the Site Layout Plan presented as Figure 2.

Stratigraphy

2.3.8 The information presented on the borehole records is consistent with the stratigraphy presented on the published geological map. From consideration of the information given on the records the expected ground conditions in the vicinity of the Site are summarised in the following table.

Formation	Base of Stratum, m bgl [m OD] ⁽¹⁾	Description
Head	0.2 to 4.0 [15.0 to 22.0]	Soft to firm orange brown slightly sandy CLAY with a little subangular to subrounded fine to coarse medium gravel of flint.
River Terrace Deposits	1.5 to 8.3 [10.8 to 20.2]	Orange brown sandy subangular to subrounded fine to coarse GRAVEL of flint.
Selsey Sand Formation ⁽²⁾	2.5 to >12.0 [<10.4 to 17.6]	Yellow/orange clayey fine to coarse SAND with occasional lenses of sandy clay.
Marsh Farm Formation	3.8 to >12.5 [<8.5 to 18.0]	Firm to stiff thinly laminated dark grey CLAY

Summary of Existing Ground Conditions

Note 1) Denotes metres below ground level {meters relative to Ordnance Datum]

2) The Selsey Sand Formation is locally absent on the northwest part of the Site where the River Terrace Deposits are directly underlain by the Marsh Farm Formation.

- 2.3.9 Details of the strata encountered are illustrated on the Schematic Geological Sections included as Figure 3 and a contour plot¹ of the base elevation of the River Terrace Deposits is included on Figure 4. The lines of the geological sections are shown on the Site Layout Plan, Figure 2.
- 2.3.10 As illustrated on the Schematic Geological Sections included as Figure 3 the railway cutting cuts through the River Terrace Deposits.

¹ Contours of the base elevation of the River Terrace Deposits have been determined from the base elevation of the River Terrace Deposits at the individual borehole locations by the Kriging geostatistical gridding method using the computer program Surfer[©] version 20.1.195.



Groundwater Conditions

- 2.3.11 Recorded groundwater levels in the standpipes installed in boreholes around the perimeter of the Site indicate groundwater levels in the River Terrace Deposits typically vary between about 2.5 and 3.0 m below ground level with the higher levels being recorded after prolonged periods of heavy rainfall. From a review of the available data the groundwater levels for February 2023 represent a complete data set of moderately conservative groundwater levels and have been selected as the base groundwater data set for use in this assessment².
- 2.3.12 The base groundwater data set indicates maximum groundwater levels of about 21.0 m OD (0.9 m depth) are present on the higher ground of the northern part of the Site. Groundwater levels fall to the southwest to about 16.5 m OD (4.5 m depth) on the central part of the Site and then to the southeast to about 13.0 m OD (3.0 m depth) on the southeast part of the Site.
- 2.3.13 Further details of the groundwater monitoring are given in a technical note on groundwater flow (Stantec, 2023). The base groundwater profile is illustrated on Schematic Geological Sections included as Figure 3 and a contour plot³ of the base groundwater levels is included on Figure 4.

2.4 **Proposed Development**

- 2.4.1 The proposed plan is to extract about 1.7 Mt of sand and gravel deposits in seven phases during a period up to 7 years. The mineral may be extracted to the base of the deposits with a maximum depth of extraction of about 7 m bgl. This corresponds to a base of working generally at 20 m OD on the higher ground of the northern part of the Site, reducing to about 15.0 OD in the in the central part of the Site and locally about 14.0 m OD along the southern and western edges of the Site.
- 2.4.2 No dewatering is planned during the extraction period and the proposed extraction is to be carried out in dry or wet conditions depending on the groundwater level at the time of excavation.
- 2.4.3 The excavated material is to be taken to a processing plant located on the northeast part of the Site. Once completed the first phase would be used for freshwater and silt lagoons to facilitate the processing of the excavated material.
- 2.4.4 The excavated void is to be infilled using in-situ soils and overburden from the Site together with imported inert materials to restore the Site to parkland and grazing land. The proposed infilling is to be carried out in dry conditions requiring the groundwater level to be artificially lowered through pumping of groundwater from the area of each phase of infilling. The infilling of each phase is to take place on a progressive basis and concurrently with the ongoing excavation of the following operational phase.
- 2.4.5 The proposed extraction phases are shown on the Site Layout Plan presented as Figure 2.

² Given that the assessment in this report considers long term drained loading conditions the use of moderately conservative groundwater levels is deemed appropriate in preference to peak groundwater levels which would be appropriate for short term loading conditons.

³ Contours of the base groundwater levels have been determined from the elevations measured in February 2023 at the individual monitoring well locations by the Kriging geostatistical gridding method using the computer program Surfer[®] version 20.1.195.

3.0 Ground Movement Assessment

3.1 Preamble

- 3.1.1 Any dewatering required to facilitate the infilling of the excavated voids in dry conditions may result in reduction of the groundwater levels in the surrounding aquifer unit and there is a potential for lower groundwater levels below the railway to the north of the Site to be observed during the proposed extraction works. Any reduction in groundwater level could increase the effective stresses in the ground and thereby could induce settlement of the ground and any supported development. The degree of settlement that could occur is dependent on the magnitude of groundwater drawdown and the geological units affected. The risk of settlement is increased for greater drawdowns and where more compressible sediments are present.
- 3.1.2 This section of the report presents (i) an assessment of the potential drawdown below the line of the railway, (ii) an assessment of the associated ground settlements, and (iii) a discussion of any potential adverse effects on the railway.

3.2 Groundwater Drawdown

- 3.2.1 To facilitate the infilling of the excavated void in dry conditions, it is expected that groundwater levels within the area of each phase of excavation may be lowered to the base of the River Terrace Deposits. On this basis the groundwater would be reduced below the base groundwater level by between about 0.5 and 3.0 m adjacent to the northern site boundary with the greater reduction along the western part of the boundary. It is understood that any dewatering may be carried out for a period of about three months during the installation of the basal and sidewall attenuation layers with groundwater being allowed to recover as the imported inert materials are placed. Elsewhere on the Site the depth of groundwater present in the River Terrace Deposits is limited and, consequently, there will be limited requirements for dewatering across the larger part of the Site.
- 3.2.2 The reduction of the groundwater levels in the surrounding aquifer unit depends on the hydraulic conductivity of the aquifer. Information on the hydraulic properties is given in a factual report on infiltration testing (Stantec 2022) and indicates the infiltration rate in the River Terrace Deposits is in the range 1×10^{-6} to 5×10^{-4} m/s.
- 3.2.3 The rate of infiltration into unsaturated soils can differ from the rate of permeation into saturated soils owing to various factors including the available non-capillary pore space, and the effects of capillary action. However, the infiltration test results indicate little change between the first test into unsaturated soil and the later tests into partially saturated soil, as such, the determined values of infiltration rate are considered representative of the expected permeability of the River Terrace Deposits.
- 3.2.4 With regard to potential effects on the railway, the most onerous conditions depend on the interrelationship of the radius of influence of the dewatering and the gradient of the drawdown curve under the railway. For the conditions on the Site, the most onerous conditions are expected to relate to the upper bound value of hydraulic conductivity and on this basis the corresponding radii of influence of the dewatering range from about 35 to 200 m for the proposed drawdown of 0.5 to 3.0 m⁴. On this basis the potential drawdown below the railway is estimated to be down to about 3.0 m.
- 3.2.5 As discussed in Section 2.4, it is expected that infilling of the excavated void will be carried out in phases. As such, the most onerous conditions relate to the dewatering of the western part of

⁴ The radius of influence has been determined using the methodology presented in Section 10 of CIRIA (1986).



Phase 1 and this assessment is limited to considering the potential effects relating to the dewatering of this cell.

3.2.6 The associated drawdown profile is shown on the Schematic Geological Sections included as Figure 3 and a contour plot of the reduced groundwater level associated with the dewatering of Phase 1W is included on Figure 4.

3.3 Effects on Network Rail Assets

Potential Effects

- 3.3.1 The reduction in groundwater level below the railway increases the effective stresses in the ground and thereby induce settlement of the railway line. Potential effects on the railway have been considered by modelling the effect of the increased effective stresses in the ground to determine the expected effects on the railway line.
- 3.3.2 The following sections of this report present the method of assessment and associated design parameters used to model the changes in effective stresses and to determine any potential ground movements, together with an assessment of any consequential effects on the railway.

Method of Assessment

- 3.3.3 Given the nature of the proposed changes in effective stress and the ground conditions the potential ground movements have been assessed using a simplified elastic half-space analysis (Oasys Pdisp) to determine the displacements and strains in the soil mass along and perpendicular to the line of the railway line resulting from the changes in effective stress.
- 3.3.4 The changes in effective stress have been modelled as an equivalent uniform load over the area of potential dewatering around and beneath the railway line. To allow for the curvature of the drawdown profile separate uniform loads have been applied for each 0.5 m increment of drawdown acting at the drawdown level. This load scenario has been used to determine the displacements along and perpendicular to the line of the railway.

Characteristic Values

- 3.3.5 It is noted that the ground investigations carried out in the area of the Site were to provide information on the available mineral resources. Consequently, information on the geotechnical properties of the ground on and in the vicinity of the Site is limited.
- 3.3.6 In the absence of site specific information, recommended characteristic values of parameters have been determined from consideration of published data based on the typical descriptions of the various strata. Given the potential duration of the proposed infilling works it is proposed to determine the effect on the railway for long-term drained loading conditions only as appropriate for assessing long-term ground movements.



3.3.7 The selected characteristic values are presented in the following table.

Summary of Selected Characteristic Values

Formation	Bulk Unit Weight ⁽¹⁾ , kN/m ³	Elastic Modulus ⁽²⁾ , MPa	Poisson's Ratio ⁽³⁾
Head	20.0	8 ^(2b)	0.3
River Terrace Deposits	20.0	30 ^(2a)	0.2
Selsey Sand formation ⁽⁴⁾	n/a	n/a	n/a
Marsh Farm Formation	20.0	15 ^(2b)	0.3

Notes:

1) Values of bulk unit weight determined from consideration of the measured values and suggested values given in Figures 1 and 2 of BS 8002 (2015)

- Values of elastic modulus are appropriate for long-term drained vertical loading conditions as determined from (a) the empirical correlation with expected SPT N values given by Stroud and Butler (1975); (b) the empirical correlation with expected undrained shear strength given by CIRIA (1983 and 2001)
- 3) Values of Poisson's ratio selected from parameters presented by Jurečič et al (2016)
- 4) The Selsey Sand Formation is absent under the northwest part of the Site and this formation has been neglected in the design analysis.
- 3.3.8 The selected values of elastic modulus represent lower bound values for an axial strain of 0.01 per cent. As such the determined deflections and distortions of the railway are deemed to be conservative.

Assessed Effects on Network ff Assets

3.3.9 The determined displacements along and perpendicular to the line of the railway resulting from the changes in effective stress associated with the proposed dewatering of the works are given in the design calculations presented in **Appendix A** of this report and are summarised in the following table.

Summary of Assessed Ground Movements

Load Scenario	Maximum	Differential Settlement, 1:n			
	Settlement, mm	Centre Alignment	Perpendicular		
LS01- Dewatering	12.5	15600	12700		

- 3.3.10 Long term differential settlements during the proposed dewatering are determined to be less than 1 vertical in 10000 horizontal. For comparison the normal limiting design values for track gradient is 1 vertical in 80 horizontal and for cross fall (cant) is 150 mm (1 vertical in 9.5 horizontal) (NR, 2021). Given that the long term differential settlements are significantly less than the normal limiting design values of the railway they are not deemed to be of concern.
- 3.3.11 On this basis it is concluded that the proposed development will not result in significant additional deflections to the Network Rail assets adjacent to the Site and the consequent risk of damage to those assets is assessed to be very low.



References

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- Stroud M A and Butler F G (1975) The Standard Penetration Test and the Engineering Properties of Glacial Materials. Proc. Symposium on the Engineering Behaviour of Glacial Materials, University of Birmingham.
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- Stantec (2023) Hamble Quarry: Groundwater Flow. Technical Note 331201108TN201, Stantec UK Ltd, Shrewsbury, Shropshire.



Guidance Notes



Essential Guidance on the Context of the Report

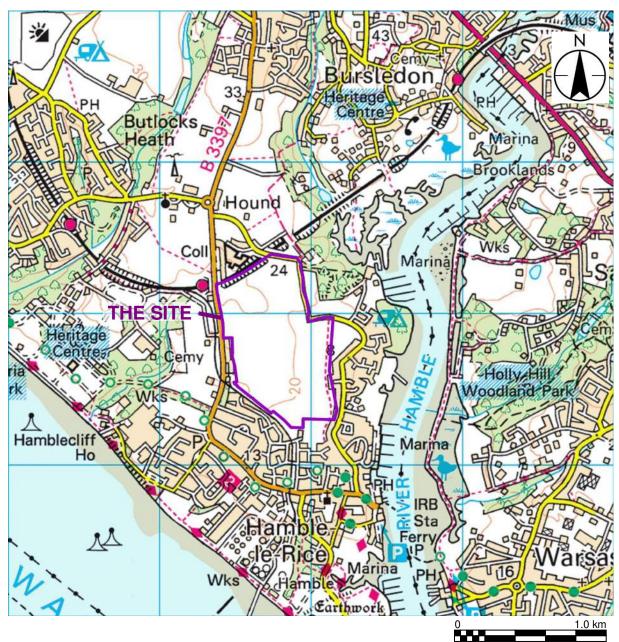
This report has been prepared within an agreed 4) timeframe and to an agreed budget that will necessarily apply some constraints on its content and usage. The remarks below are presented to assist the reader in understanding the context of this report and any general limitations or constraints. If there are any specific limitations or constraints, they are 5) described in the report text.

- The opinions and recommendations expressed in 1) this report are based on statute, guidance, and appropriate practice current at the date of its preparation. Stantec UK Ltd (Stantec) does not accept any liability whatsoever for the consequences of any future legislative changes or the release of subsequent guidance documentation, etc. Such changes may render some of the opinions and advice in this report inappropriate or incorrect and we will be pleased to advise if any report requires revision due to changing circumstances. Following delivery of the report Stantec has no obligation to advise the Client or any other party of such changes or their 7) repercussions.
- 2) Some of the conclusions in this report may be based on third party data. No guarantee can be given for the accuracy or completeness of any third party data used.
- 3) The conclusions and recommendations made in this report and the opinions expressed are based on the information reviewed and/or the ground conditions encountered in exploratory holes and the results of any field or laboratory testing undertaken. There may be ground conditions at the site that have not been disclosed by the information reviewed or by the investigative work undertaken. Such undisclosed conditions cannot be taken into account in the reporting.

- It should be noted that groundwater levels, groundwater chemistry, surface water levels, surface water chemistry, soil gas concentrations and soil gas flow rates can vary due to seasonal, climatic, tidal and man made effects.
- Unless specifically stated to the contrary, this report does not purport to be a "Geotechnical Design Report" as defined in Clause 2.8 of Eurocode 7 (Geotechnical Design BS EN 1997-1:2004).
- 6) This report has been written for the sole use of the Client stated at the front of the report in relation to a specific development or scheme. The conclusions and recommendations presented herein are only relevant to the scheme or the phase of project under consideration. This report shall not be relied upon or transferred to any other party without the express written authorisation of Stantec. Any such party relies upon the report at its own risk.
- 7) The interpretation carried out in this report is based on scientific and engineering appraisal carried out by suitably experienced and qualified technical consultants based on the scope of our engagement. We have not taken into account the perceptions of, for example, banks, insurers, other funders, lay people, etc, unless the report has been prepared specifically for that purpose. Advice from other specialists may be required such as the legal, planning and architecture professions, whether specifically recommended in our report or not.
- Public or legal consultations or enquiries, or consultation with any Regulatory Bodies (such as the Environment Agency, Natural England or Local Authority) have taken place only as part of this work where specifically stated.



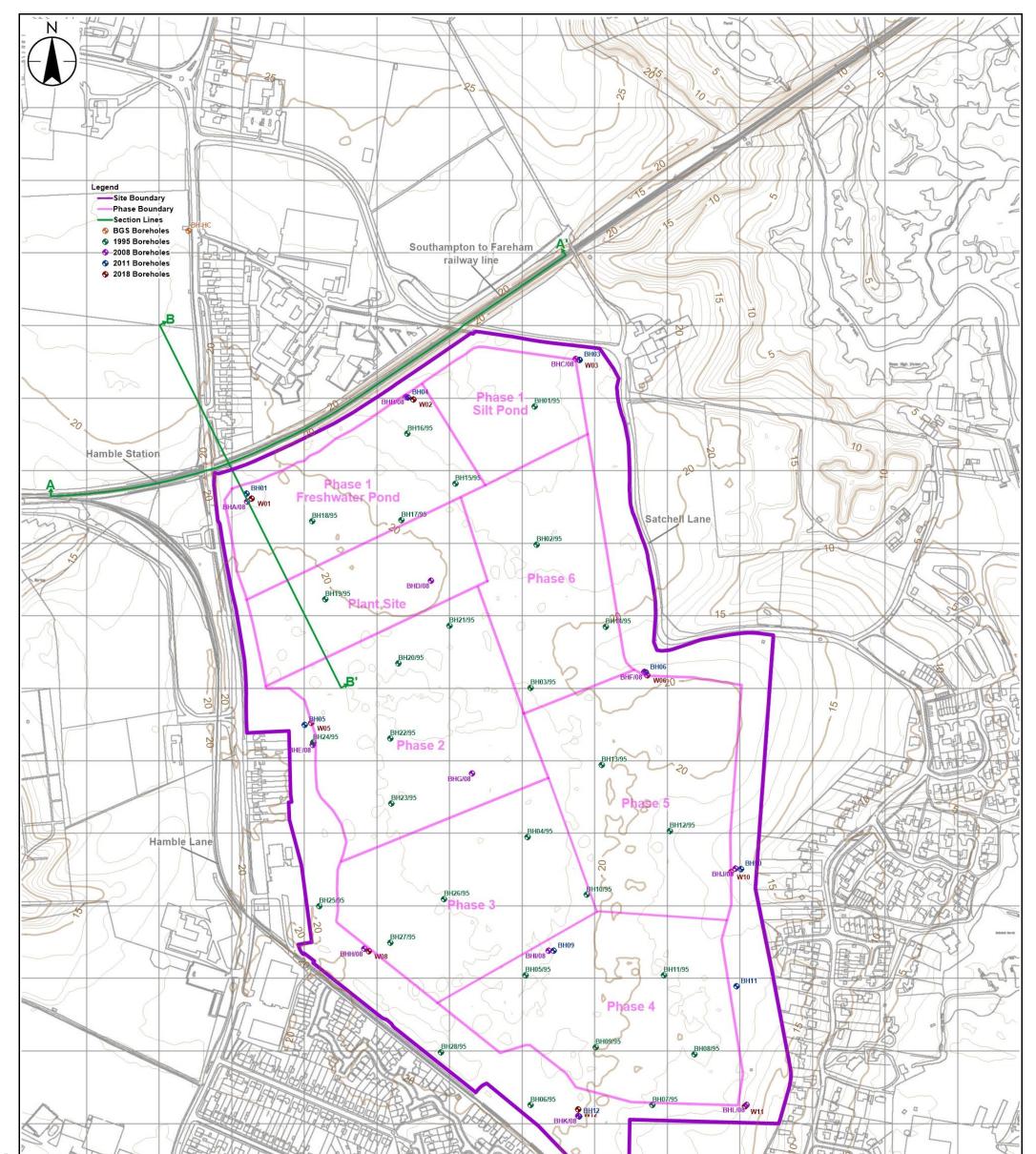
Figures



Scale 1:25000

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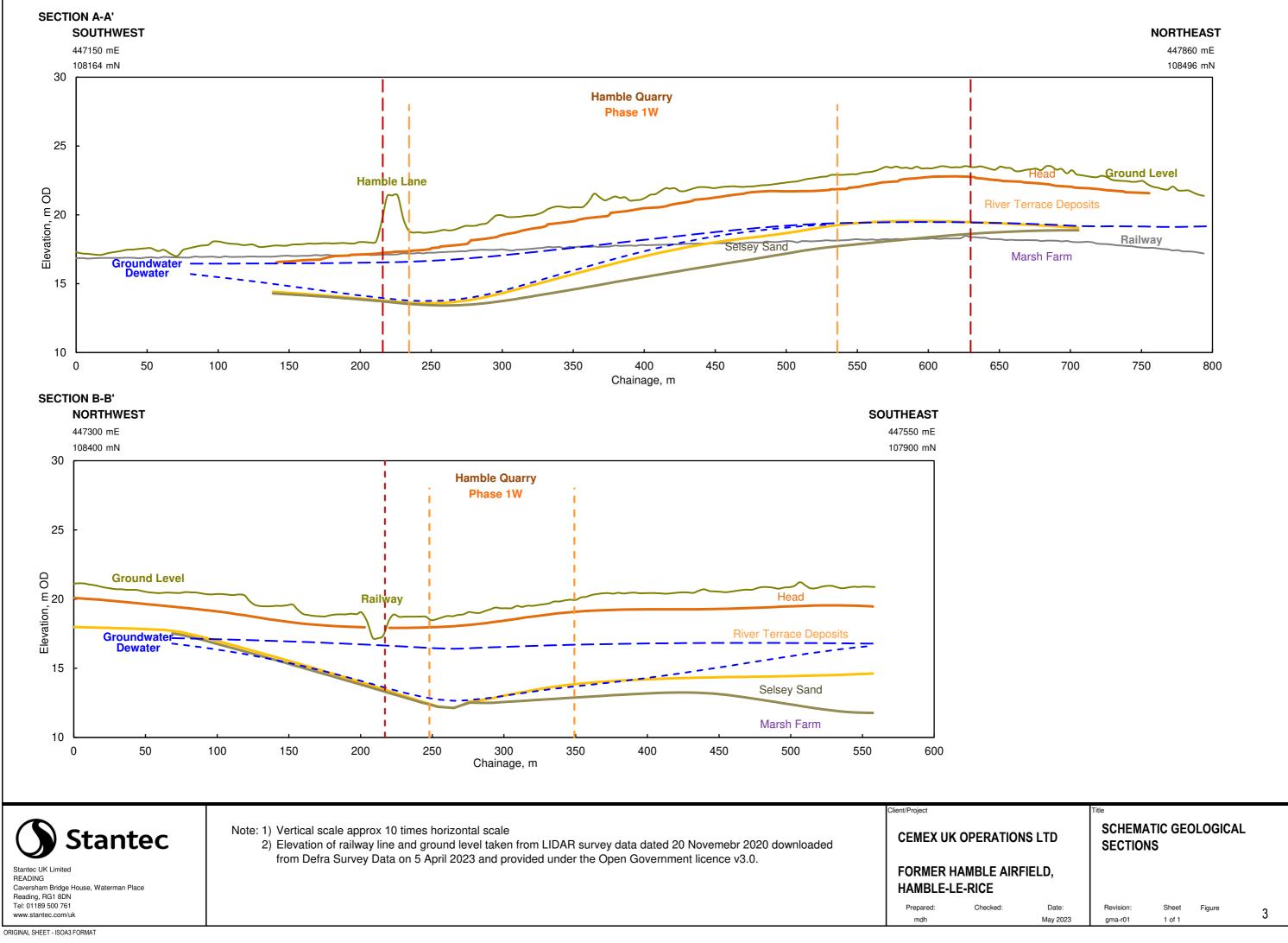


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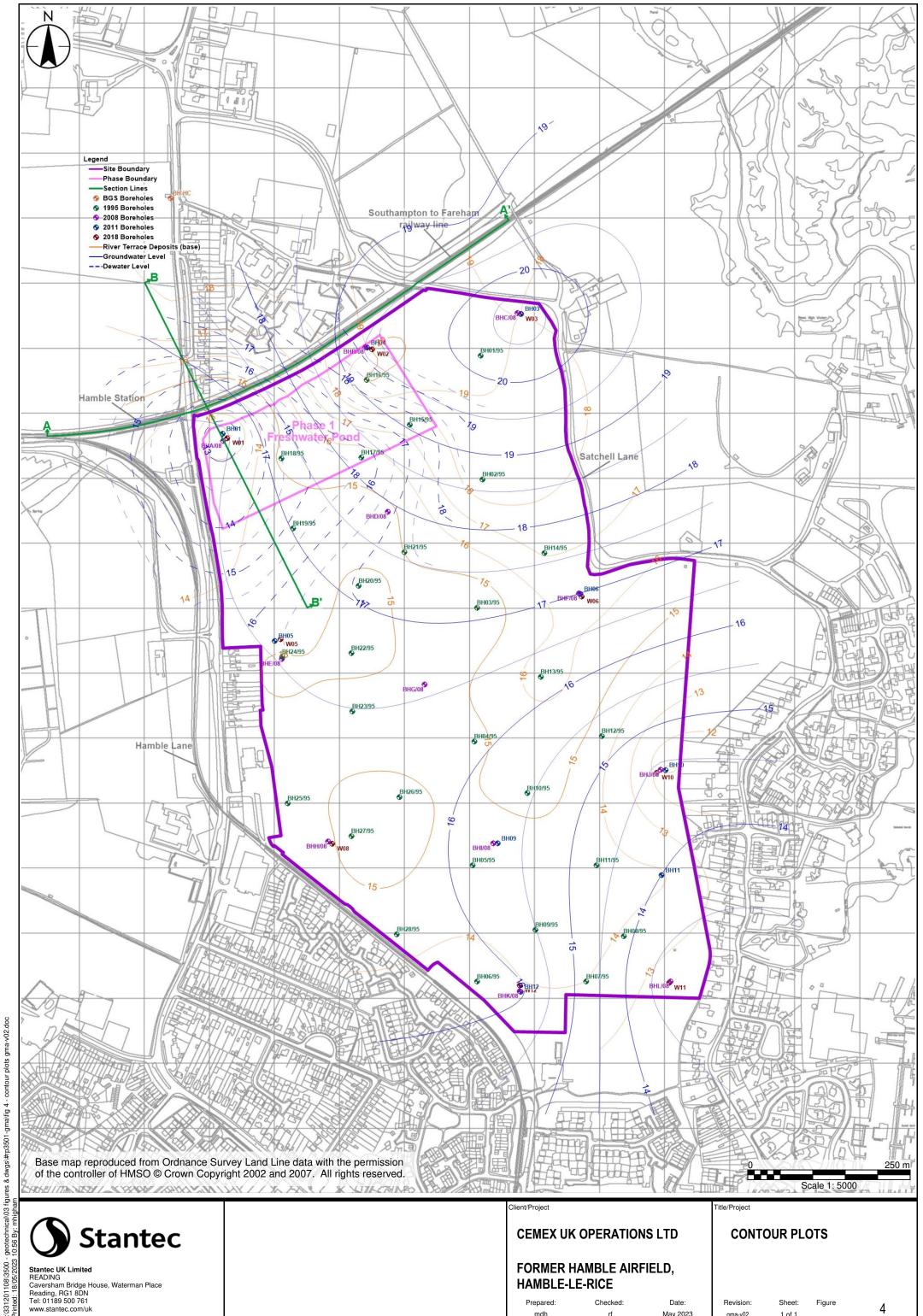
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Appendix A: Ground Movement Calculations

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GROUND MOVEMENT ASSESSMENT - NETWORK RAIL ASSETS



Project Title	FORMER HAMBLE AIRFIELD, HAMBLE-LE-RICE							
Project No	331201108	3500	Ву	mdh	Chkd	rf	Date	18/05/2023

Existing Conditions

The Site is situated on a ridge between the valley of the River Hamble to the east and Southampton Water to the West and comprises undeveloped rough grassland partly colonised by dense scrub of the landing areas of the former Hamble North Airfield. Natural ground levels across the Site are generally between about 20.0 and 21.0 m relative to Ordnance Datum (OD) rising to about 23.0 m OD on the northern part of the Site and locally falling to about 15.0 m OD along the eastern boundary of the Site.

Proposed Works

The proposed works comprise the extraction of sand and gravel. No dewatering is planned and the extraction is to be carried out in dry or wet conditions depending on the groundwater level at the time of excavation.

The excavated void is to be infilled using in-situ soils and overburden from the Site together with imported inert materials to restore the Site to parkland and grazing land. The proposed infilling is to be carried out in dry conditions requiring the groundwater level to be artificially lowered through off-site pumping of groundwater from the area of each phase of infilling.

The proposed dewatering will result in reduction of the groundwater levels in the surrounding aquifer including below the Network Rail asset to the north of the Site Any reduction in groundwater level will increase the effective stresses in the ground and can thereby induce settlement of the ground and any supported development.

The Network Rail asset comprises the Hamble to Burlesdon section of the Southampton to Fareham railway which is aligned approximately southwest-northeast parallel with the northern site boundary. The railway comprises two tracks running in cutting down to about 3.0 m below the adjacent ground level. The base of the cutting is about 7 m wide, and the side slopes are formed at gradients of about 1 vertical in 6 horizontal.

Ground Conditions

Chrono	Base Level	Description
Strata	m bgl	
Head	0.2 to 4.0	Soft to firm slightly sandy CLAY with a little gravel.
River Terrace Deposits	1.5 to 8.3	Sandy fine to coarse GRAVEL of flint
Selsey Sand Formation	1.8 to 9.5	Clayey fine to coarse SAND
Marsh Farm Formation	>20.0	Firm to stiff thinly laminated CLAY

The Selsey Sand Formation typically sub-crops on the southwest part of the Site and locally on the higher ground on the northern part of the Site. Across a central band from the northwest to southeast of the Site the Selsey Sand Formation is generally absent.

Groundwater Conditions

Groundwater levels of about 21.0 m OD (0.9 m depth) are present on the higher ground of the northern part of the Site falling to the southwest to about 16.5 m OD (4.5 m depth) on the central part of the Site and then to the southeast to about 13.0 m OD (3.0 m depth) on the southeast part of the Site.

Assumptions

1) The most onerous conditions relate to the dewatering of the western part of Phase 1.

2) Groundwater levels in the area of the dewaterng will be lowered by 0.5 to 3.0 m to the base of the River Terrace Deposits and the associated radius of influence of the dewatering works will extend between 35 and 200 m from the edge of the workings.

3) The changes in effective stress owing to the reduction in groundwater levels can be modelled as an equivalent uniform load over the plan area of the dewatering with separate uniform unloads each 0.5 m of drawdown acting at the drawdown level to allow for the curvature of the drawdown profile.

4) The existing ground conditions can be modelled as a linear elastic soil mass.

Sheet	01	of	05		Ref	HQ-GMA01	Ver	01
J·\3312011()8\3500 - Ge	eotechnical\()4 Data\Eng	+Calcs\gma\[nr railway r01 xls]Design				

GROUND MOVEMENT ASSESSMENT - NETWORK RAIL ASSETS



Project Title	FORMER HAMBLE	AIRFIELD, H	AMBLE-	LE-RICE				
Project No	331201108	3500	Ву	mdh	Chkd	rf	Date	18/05/2023

Assumptions - Cont'd

5) The stresses in the soil mass can be calculated assuming values of elastic modulus and Poisson's ratio are constant throughout the soil mass.

6) The vertical strains, and thereby displacements, can be calculated using selected values of drained vertical elastic modulus and Poisson's ratio for each soil layer.

7) The railway track experiences the same movements as the supporting soils.

Ground Properties

Strata	Base of Stratum m bgl [m OD}	Bulk Unit Wt, γ kN/m ³	Elastic Modulus MPa [@m OD]	Poisson's Ratio
Head	1.1 [17.7]	20.0	8.0	0.3
River Terrace Deposits	5.6 [13.2]	20.0	30.0	0.2
Selsey Sand Formation ⁽¹⁾	n/a	n/a	n/a	n/a
Marsh Farm Formation	>20.0 [<-1.2]	20.0	15.0	0.3

(1) The Selsey Sand Formation is absent under the northwest part of the Site and this formation has been neglected in the design analysis.

Load Scenarios

Consider a single load scenario

1) Changes in effective stress associated with the reduction in groundwater level resulting from the dewatering of the proposed works.

Method of Analysis

Consider settlement of soil mass associated with the proposed dewatering using Oasys Pdisp v19.2.
 Model reduction in groundwater as equivalent uniform loads over the plan area of the dewatering in accordance with Assumption (3).

3) Determine the ground movements on a line along (i) the centre alignment of the railway, and (ii) perpendicular to the railway through the area of the proposed dewatering.

Load	Description	Load level	Loade	d Area	Pressure
Case		m OD	m	m	kPa
LC01	Dewatering				
	3.5	13.2	50.0	50.0	5.0
	3.0 to 3.5	13.7	100.0	120.0	5.0
	2.5 to 3.0	14.2	190.0	180.0	5.0
	2.0 to 2.5	14.7	280.0	240.0	5.0
	1.5 to 2.0	15.2	350.0	300.0	5.0
	1.0 to 1.5	15.7	420.0	360.0	5.0
	0.5 to 1.0	16.2	490.0	420.0	5.0

Design Loads

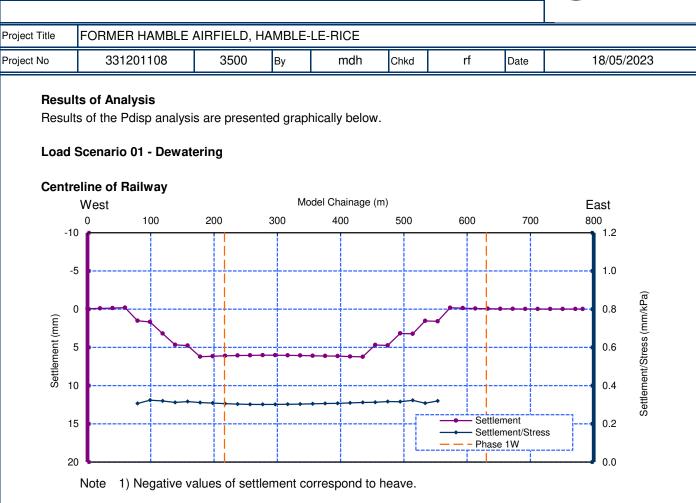
Design Note

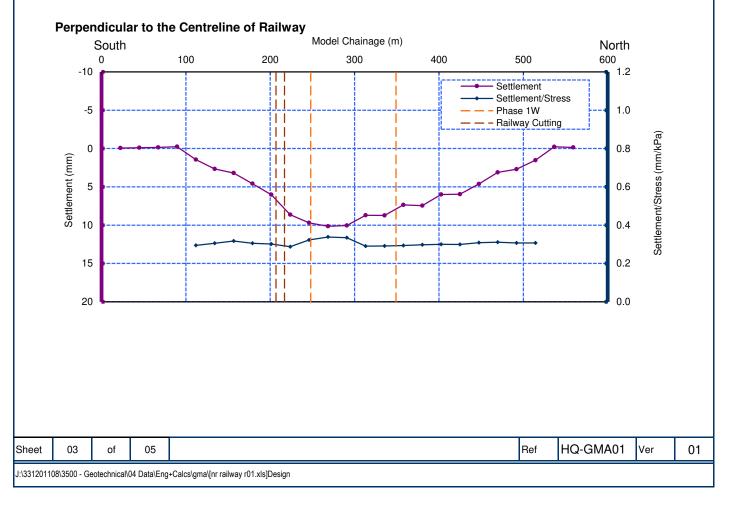
In Pdisp the dewatering profile is modelled with step-changes between the differing stresses changes. The use of step-changes results in unrealistic settlement profiles and consequently determined differential settlements at the location of the step-changes. To mitigate this effect of the modelling, the results of the Pdisp analysis have been used to determine the ratio of settlement to applied vertical stress with the settlement profile and corresponding differential settlements then determined from the profile of ground water reduction determined from the ground model. Given the large plan area of the loaded areas this assumption is deemed accepatble as the settlements will be predominantly one-dimensional with limited lateral dispersion of the applied vertical stresses.

Sheet	02	of	05		Ref	HQ-GMA01	Ver	01
J:\3312011	08\3500 - Ge	eotechnical\	04 Data\Eng	+Calcs\gma\[nr railway r01.xls]Design				

GROUND MOVEMENT ASSESSMENT - NETWORK RAIL ASSETS

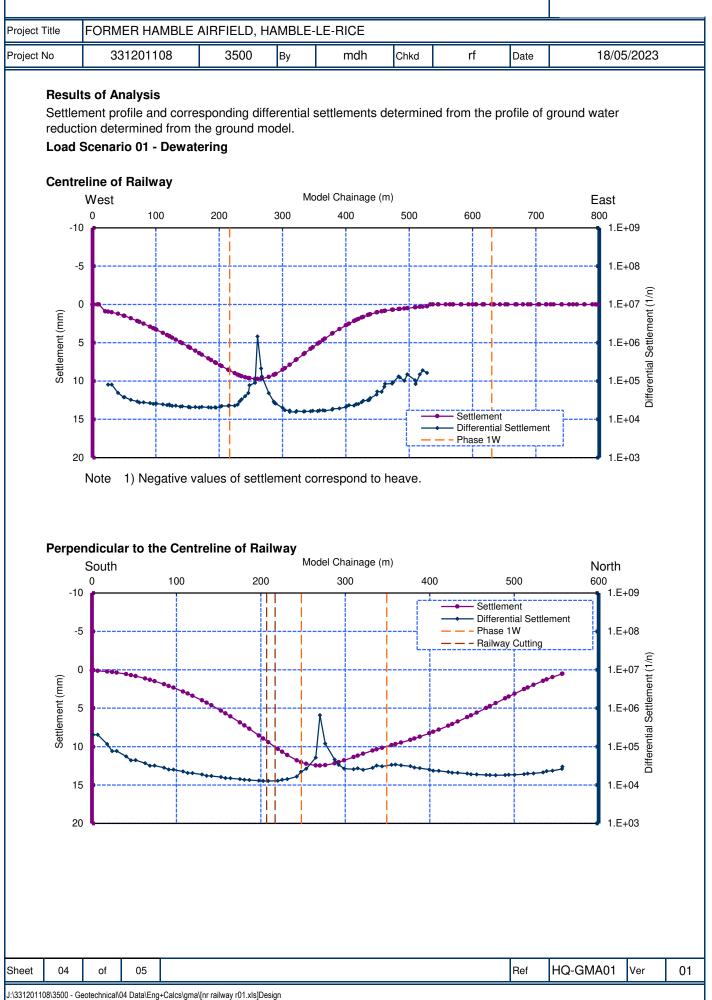






GROUND MOVEMENT ASSESSMENT - NETWORK RAIL ASSETS





GROUND MOVEMENT ASSESSMENT - NETWORK RAIL ASSETS



Project Title	FORMER HAMBLE	AIRFIELD, HA	MBLE-I	LE-RICE					
Project No	331201108	3500	Ву	mdł	n Chkd	rf	Date	18/05/2023	
Sum Deter	nary mined ground moveme	ents under the	NR ass	sets are	summarise	d in the followi	ng table		_
	Load Scenario Max Settlement, Δδ Maximum Differential, $\Delta L/\Delta \delta$								
		(m	ım)		Centre Alignment			Perpendicular	
	LS01	1:	2.5		1	5600		12700]

Results of analysis indicate

Differential settlements associated with the dewatering of the proposed works are determined to be less than 1 vertical in 10000 horizontal and are therefore deemed to be negligible .

On this basis it is concluded that the proposed development will not result in significant displacements that cannot be accommodated by the Network Rail assets in the vicinity of the Site.

Sheet	05	of	05		Ref	HQ-GMA01	Ver	01
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\bigcirc	PETER BRETT	Job No.	Sheet No.	Rev.
Oasys	ASSOCIATES -READING	331201108		
Former Hamble Airfield, Ham	ble-le-Rice	Drg. Ref.		
Ground Movement Assessme	ent - Network Rail Assets			
Estimated Ground Movement	s - Load Scenario 01	Made by mdh	Date 08-May-2023	Checked

Analys Global Maximu Horizo Displa	m allowabl ntal rigic cements at		el: -1.20 is calculat	[m OD]	. 0	
		Number of intermediate displacement levels		odulus	Poissons ratio	Non-linear curve
	[mOD]		Top [kN/m ²]	Btm [kN/m ²]		
1	18.800	2	8000.0		0.30000	None
2		9	30000.			
3	13.200	29	15000.	70000.	0.30000	None
Soil Z	Zones				_	rofile
Zone	Name	X coordinates		ordinates	s P	rorrie
	Name	X coordinates min max [m] [m] 0.0 800.	min [m]	max [m]	.00 Soil	

Non-linear Curve Coordinates - Non-linear Curve 1

Load Data

Load	Name			L	oaded pla	ne				L	oads		
ref.		Orientation		tre of 1	oad	Angle of local x	Shape		nsion		Load valu		Number
				(Global)		w.r.t. global X		Width x/ Depth y		Normal z	Tange	ntial	of
			х	Y	Z(level)			Radius			x	У	rectangles
			[m]	[m]	[m]	[Degrees]		[m]	[m]	[kN/m²]	[kN/m²]	[kN/m²]	
	1 DW3.5	Horizontal	325.00	360.00	13.200	26.600	Rectangular	50.000	50.000	5.0000	0.0	0.0	N/A
	2 DW3.0	Horizontal	330.00	350.00	13.700	26.600	Rectangular	100.00	120.00	5.0000	0.0	0.0	N/A
	3 DW2.5	Horizontal	350.00	340.00	14.200	26.600	Rectangular	190.00	180.00	5.0000	0.0	0.0	N/A
	4 DW2.0	Horizontal	370.00	340.00	14.700	26.600	Rectangular	280.00	240.00	5.0000	0.0	0.0	N/A
	5 DW1.5	Horizontal	375.00	340.00	15.200	26.600	Rectangular	350.00	300.00	5.0000	0.0	0.0	N/A
	6 DW1.0	Horizontal	380.00	340.00	15.700	26.600	Rectangular	420.00	360.00	5.0000	0.0	0.0	N/A
	7 DW0.5	Horizontal	385.00	340.00	16.200	26.600	Rectangular	490.00	420.00	5.0000	0.0	0.0	N/A

Displacement Data

Ref.	Туре	Name	Direction of		Li: irst point		or extrusi S	on econd poin		No. of intrvls across	Extrusion	No. of intrvls along	Calculate	Show Detailed
			Extrusion	x	Y	Z(level)	x	Y	Z(level)	extrusion/line	Depth	extrusion		results
				[m]	[m]	[m]	[m]	[m]	[m]		[m]			
1	Line	NR Railway	N/A		300.00000						N/A	N/A	Yes	No
2	Line	NR Railway2	N/A		300.00000						N/A	N/A	Yes	No
3	Line	X-Section	N/A	200.00000	600.00000	18.80000	450.00000	100.00000	18.80000	125	N/A	N/A	Yes	No
4	Line	X-Section2	N/A	200.00000	600.00000	13.20000	450.00000	100.00000	13.20000	125	N/A	N/A	Yes	No

RESULTS FOR GRIDS

Analysis: Boussinesq Global Poisson's ratio: 0.20 Horizontal rigid boundary level: -1.20 [m OD]

The maximum displacement difference between Boussinesq method = 1.5417mm and Mindlin method = -8.7114mm occurs at point X = 527.27m Y = 538.64mLevel = 18.800mOD and is: 10.253mm

	Location Stresses										
Name	х	Location Y	Z[Level]	z	Calc	Stre Vert		Vent			
	x	ĭ	z[revei]	2	Level	Stress	Sum Princ	Vert Strain			
	[m]	[m]	[mOD]	[mm]	[mOD]	[kN/m ²]	[kN/m ²]	[%]			
			13.20000	8.8907	13.030	35.000		0.13167			
		350.00000	13.70000	9.2427	13.450	30.000	71.356	0.072429			
		340.00000		8.2240	13.950	25.000		0.060257			
		340.00000	14.70000	8.3974	14.450	20.000	47.775	0.048150			
		340.00000		8.5910	14.950	15.000		0.036079			
		340.00000	15.70000	8.7124	15.450	10.000	23.950	0.024033			
		340.00000		8.6941	15.986	5.0000		0.012007			
NR Railway		300.00000 301.76768	18.80000	-0.065705	18.617	0.0		0.0			
		303.53535	18.80000	-0.074690	18.617	0.0		0.0			
		305.30303		-0.079912	18.617			0.0			
		307.07071	18.80000	-0.085721	18.617	0.0		0.0			
		308.83838		-0.092209	18.617	0.0		0.0			
		310.60606	18.80000	-0.099486	18.617	0.0		0.0			
	74.74747	312.37374	18.80000	-0.10768	18.617	0.0		0.0			
	78.28283	314.14141	18.80000	-0.11694	18.617	0.0		0.0			
	81.81818	315.90909	18.80000	-0.12743	18.617	0.0	0.0	0.0			
		317.67677	18.80000	-0.13932	18.617	0.0		0.0			
		319.44444		-0.15270	18.617	0.0		0.0			
		321.21212	18.80000	-0.16751	18.617	0.0		0.0			
		322.97980		-0.18318	18.617			0.0			
		324.74747	18.80000	-0.19776	18.617	0.0		0.0			
			18.80000	-0.20568	18.617			0.0			
		328.28283	18.80000	-0.19095	18.617	0.0		0.0			
			18.80000	-0.10268	18.617	0.0		0.0			
		331.81818	18.80000	1.2805	18.617	0.0		0.0			
		335.35354	18.80000	1.5061	18.617	0.0		0.0			
		337.12121		1.5537	18.617	0.0		0.0			
		338.88889	18.80000	1.5531	18.617	0.0		0.0			
		340.65657		1.5445	18.617	0.0		0.0			
		342.42424	18.80000	1.5593	18.617	0.0		0.0			
		344.19192		1.6699	18.617	0.0		0.0			
		345.95960	18.80000	2.2622	18.617	0.0		0.0			
			18.80000	3.0585	18.617			0.0			
	148.98990	349.49495	18.80000	3.1887	18.617	0.0	0.0	0.0			
			18.80000	3.2068	18.617			0.0			
		353.03030	18.80000	3.1952	18.617	0.0		0.0			
			18.80000	3.1845	18.617	0.0		0.0			
	163.13131	356.56566	18.80000	3.2075	18.617	0.0		0.0			
		358.33333		3.3624	18.617	0.0		0.0			
		360.10101	18.80000	4.2483	18.617	0.0		0.0			
		361.86869		4.6560	18.617	0.0		0.0			
		363.63636	18.80000	4.7308	18.617	0.0		0.0			
		365.40404 367.17172	18.80000	4.7349	18.617	0.0		0.0			
			18.80000	4.7219 4.7166	18.617			0.0			
		370.70707	18.80000	4.7561	18.617	0.0		0.0			
			18.80000		18.617			0.0			
		374.24242	18.80000	5.9506	18.617	0.0		0.0			
			18.80000	6.1811	18.617			0.0			
		377.77778	18.80000	6.2243	18.617	0.0	0.0	0.0			
	209.09091	379.54545		6.2229	18.617	0.0		0.0			
		381.31313	18.80000	6.2097	18.617	0.0		0.0			
		383.08081		6.1959	18.617	0.0		0.0			
	219.69697	384.84848	18.80000	6.1857	18.617	0.0		0.0			
		386.61616	18.80000	6.1779	18.617	0.0		0.0			
		388.38384	18.80000	6.1687	18.617	0.0		0.0			
		390.15152		6.1571	18.617	0.0		0.0			
		391.91919		6.1442	18.617	0.0		0.0			
		393.68687		6.1313	18.617	0.0		0.0			
		395.45455	18.80000	6.1190	18.617	0.0		0.0			
	244.44444		18.80000	6.1075 6.0970	18.617	0.0	0.0	0.0			

a ca 10	PETER BRETT	Job No.
asys	ASSOCIATES -READING	33120110

Former Hamble Airfield, H	amble-le-Rice
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Ground Movement Assessment - Network Rail Assets Estimated Ground Movements - Load Scenario 01

Job No.	Sheet No.	Rev.
331201108		
Drg. Ref.		
Made by mdh	Date 08-May-2023	Checked

Location Y Stresses : Sum Princ Calc Level [mOD] 18.617 18.617 Vert Stress [kN/m²] Vert Strain [%] х Z[Level] [kN/m²] [m] [m] [mm] [mOD]
 [m]
 [m]
 [mOD]

 251.51515
 400.75758
 18.80000

 255.05051
 402.52525
 18.80000

 258.58586
 404.29293
 18.80000

 256.251.2512
 406.60661
 18.80000

 256.5557
 407.82282
 18.80000

 256.5657
 407.82282
 18.80000

 272.72727
 111.3634
 18.80000

 276.62623
 131.3131
 18.80000

 278.33334
 14.89899
 18.80000
 mm] 6.0873 6.0784 6.0703 6.0630 6.0564 6.0505 6.0454 6.0454 6.0409 6.0371 6.0339 0.0 0. 18.617 18.617 18.617 18.617 18.617 18.617 18.617 18.617 18.617 0.0 0.0 0.0 276.26263 413.13131 18.80000 279.73798 414.89899 18.80000 283.3333 416.66667 18.80000 290.40404 420.2020 18.80000 290.40404 420.2020 18.80000 297.47475 423.73737 18.80000 297.47475 423.73737 18.80000 304.54545 427.27273 18.80000 304.54545 427.27273 18.80000 311.61616 430.80008 18.80000 315.51552 429.04040 18.80000 315.51552 429.04041 8.80000 315.51552 432.57576 18.80000 313.66687 434.34433 18.80000 322.22222 433.67479 18.80000 322.22222 433.67479 18.80000 333.63634 443.18182 18.80000 334.64674 443.18485 18.80000 335.65505 444.8485 18.80000 336.65697 444.8485 18.80000 336.65697 444.8485 18.80000 334.43434 44.71171 18.80000 334.43434 44.71171 18.80000 335.55555 35.22525 18.80000 354.0404 452.22222 18.80000 354.10404 452.22225 18.80000 355.5555 43.78785 18.80000 354.10404 452.22225 18.80000 355.5555 43.78785 18.80000 354.10404 452.2225 18.80000 354.1111 455.35555 18.80000 0.0 0.0 .0339 .0312 .0292 18.61 18.61 18.61 6.0292 6.0277 6.0268 6.0263 6.0264 6.0271 6.0282 6.0298 6.0298 18.617 18.617 18.617 18.617 18.617 18.617 18.617 18.617 0.0 0.0 0.0 0.0 0.0 0.0 .0320 18.61 18.61 18.61 0.0 6.0377 6.0413 6.0452 6.0494 6.0539 6.0586 6.0636 6.0688 18.617 18.617 18.617 18.617 18.617 18.617 18.617 18.617 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 350.5055
 450.2223
 10.0000

 354.04040
 452.02202
 18.80000

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 455.5556
 18.80000

 361.1111
 455.5556
 18.80000

 364.04040
 452.02202
 18.80000

 364.111
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 18.80000

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 18.80000

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 556.31313

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 0.0 0.0 18.80000 18.61 18.80000 18.80000 18.80000 18.80000 18.80000 18.80000 0.0 0.0 1.0 18.617 18.617 18.617 -0.16030 -0.14644 -0.13353 0.0 0.0 -0.12194 0.0 18.61 0.0 -0.12194 -0.11168 -0.10262 -0.094623 -0.087543 -0.087543 -0.070605 -0.070605 -0.066077 -0.061986 -0.058275 -0.054898 0.0 0.0 18.80000 18.61 18.80000 18.80000 18.80000 18.80000 18.80000 18.80000 18.80000 18.80000 18.80000 18.617 18.617 18.617 18.617 18.617 18.617 18.617 18.617 18.617 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
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 18.80000

 665.15125
 601.61616
 605.80808
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 672.2222
 611.4114
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 662.5125
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 672.2222.611.11111
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 662.434991
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 672.2222.6141.81141
 18.80000
 662.4349491
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 673.2222.218.612.4124
 18.80000
 707.57576
 22.7273
 18.80000

 704.50402.712202
 $\begin{array}{c} -0.051814\\ -0.048090\\ -0.048090\\ -0.046039\\ -0.04070\\ -0.039750\\ -0.039750\\ -0.037853\\ -0.037853\\ -0.0327853\\ -0.0328445\\ -0.032445\\ -0.032445\\ -0.032445\\ -0.032445\\ -0.0226873\\ -0.026873\\ -0.026873\\ -0.026873\\ -0.026873\\ -0.026873\\ -0.026873\\ -0.0268$ 18.617 0.0 0.0 0.0 18.617 18.617 18.617 18.617 18.617 18.617 18.617 18.617 0.0 0.0 0.0 0.0 18.617 18.617 18.617 18.617 18.617 18.617 18.617 18.617 18.617 18.617 18.617 18.617 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 18.617 18.617 18.617 0.0 0.0 0.0 18.61 18.61 -0.017679 -0.017093 -0.016535 18.61 18.61 18.61 0.0 0.0 -0.016002 18.617 0.0 0.0 0.0 -0.013493 -0.015007 -0.014542 -0.014097 -0.013671 -0.013263 18.617 18.617 18.617 0.0 0.0 0.0 0.0 0.0 0.0 0.0 18.617 0.0 0.0

PETER BRETT	Job No.	Sheet No.	Rev
ASSOCIATES -READING	331201108		
le-le-Rice	Drg. Ref.		

Former Hamble Airfield, Hamble-le-Rice Ground Movement Assessment - Network Rail Assets Estimated Ground Movements - Load Scenario 01

Z[Level]

[mOD]

18.80000

13.2000 13.20000 13.20000 13.20000 13.20000 13.20000

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Location

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[m] [m] 750.0000 650.0000 53.5353 301.76768 57.07071 303.55535 60.6066 305.30303 64.14141 307.07071 67.67677 308.83838 71.21212 310.60606 74.74747 312.37374 78.28283 314.14141

74,74147,312,37374 78,2828,314,14141 81,81818,315,90909 85,35354,317,67677 88,88889,319,44444 92,42424,321,21212 95,95960,322,97980 99,49495,324,74747 103,03030,326,51515 100,55566,328,28283 110,10101,330,05051 113,63636,311,81818

103.03303 246.51313 103.03430 248.24283 110.10101 330.05051 113.03456 331.811816 120.70707 335.8354 120.70707 335.8354 124.24242 37.12212 127.77778 338.88889 131.31313 340.65657 134.84848 342.42424 138.38348 441.9192 141.9191 345.95960 145.45455 351.26263 155.05061 333.0330 155.95956 347.72727 148.98990 349.49495 155.95956 347.7318 155.95956 347.7318 155.95956 347.97292 166.66667 353.0333 170.20202 360.10101 173.73737 363.65666 166.66667 171.27273 363.65366 177.47273 363.6536 173.434343 367.17173 184.4449 372.47475 198.48449 372.47475 198.48459 374.24425 202.0220 376.01010 124.26459 374.24425 202.55556 347.29728 203.90991 379.54545 212.62626 381.3131 216.16162 383.08081 215.95595 348.8888 233.30303 390.15152 233.8388 31.91919 237.37374 395.64687 240.955556

233,83838 391,91919 237,37374 393,66687 240,90909 395,45455 244,4444 397,22222 247,97980 398,98990 251,51515 400,75758 255,05051 402,52525 258,58586 404,29293 262,12121 406,06061 265,65657 407,82828 269,19192 409,55596 272,72727 411,36364

72.72727 411.36364 76.26263 413.13131 79.79798 414.89899

279.79798 414 89899 279.79798 414 89899 283.3333 416.6667 286.8669 418.43434 290.4004 420.2020 293.93939 421.96670 297.47475 423.73737 301.01010 425.50505 308.80801 427.27273 308.80801 427.27273 311.61616 430.80808 315.15152 432.57576 318.68687 443.34343 322.22222 436.11111 325.75758 447.87879 329.2293 439.64646

329.29293 439.64646 332.82828 441.41414 336.36364 443.18182 339.89899 444.94949 343.4343 446.7117 346.96970 448.44845 350.50505 450.25253 34.04040 452.02020 357.57576 453.78788 361.11111 455.55556 364.64646 457.3223 364.8454 450.00051

 $\begin{array}{c} 361.1111 \ 455.5556\\ 346.46466 \ 457.32232\\ 368.18182 \ 459.09091\\ 317.17171 \ 460.85859\\ 375.25253 \ 462.26262\\ 375.25253 \ 462.462.993\\ 382.32223 \ 466.16162\\ 385.85559 \ 467.92292\\ 383.33394 \ 469.6987\\ 322.9229 \ 471.46465\\ 336.46465 \ 473.23232\\ 410.60606 \ 480.3033\\ 410.60606 \ 480.3033\\ 410.60606 \ 480.3033\\ 410.414.1414 \ 42.07071\\ 477.67677 \ 483.8838\\ 421.21212 \ 485.60666\\ \end{array}$

421.21212 485.60606 424.74747 487.37374 428.28283 489.14141

424, 4147, 427, 437, 437, 437, 4487, 437, 4487, 437, 4487, 437, 4487, 437, 4487, 450

191.91919 520.95960 195.45455 522.72727 198.98990 524.49495

498.98990 524.49495 502.52525 526.26263 506.06061 528.03030 509.59596 529.79798 513.13131 531.56566 516.66667 533.3333 520.20202 535.10101 523.73737 536.86869

z

[mm]

[mm] -0.012872 -0.064180 -0.068339 -0.072928 -0.072928 -0.078009 -0.083657 -0.089961 -0.097025 -0.10497 -0.11394 -0.12408 -0.13553 -0.14838

-0.13553 -0.14838 -0.16250 -0.17725 -0.19054 -0.19654 -0.17868 -0.085327

0.27056 0.89023 1.1163 1688 1716

1846 .1. .3007 .7910 .3390 .74f

2.3390 2.4746 2.4962 2.4870 2.4786 2.5046 2.6645 3.3309 3.7232

8.8017 8.8079 8.7964 8.7924 8.8340 1.0658 1.8538

0885 1335 1330

.1204 .1071 .0973 .0898

.0809 .0696 .0569

0441 0319 0206

.0101

.0005 .9917 .9837 .9764 .9699 .9640

.9589 .9545 .9507

.9475 .9449 .9429 .9415 .9405 .9401 .9402

.9402 .9408 .9420 .9436 .9458 .9458 .9484 .9515 .9550

.9589 .9631 .9676 .9724 .9773 .9825 .9879 .9935 .9993 .0054

.0054 .0118 .0186 .0257 .0327 .0387

0424 0426 0411

.0411 .0423 .0474 .0551 .0643 .0740 .0843 .0950

5.1421 5.1492 5.1398 5.0738

.7572 .9868 .8031

7673

.7869 .8052 .8183 .8117 .7403 .4015 .6907 .4962

2.4586 2.4641 2.4831 2.5041 2.5187 2.5104 2.4294 2.0735 1.4156 1.2094

Stresses Sum Princ

[kN/m²]

Vert Strain [%]

0.0 0.0 0.17338 -239.85E-6

 $\begin{array}{c} 0.17338-239.885-c\\ 0.18568-226.815-6\\ 0.19944-275.788-6\\ 0.21439-277.108-6\\ 0.21439-277.108-6\\ 0.22523-348.798-6\\ 0.25253-348.798-6\\ 0.33251-417.288-6\\ 0.3342.9-460.568-6\\ 0.342420-460.568-6\\ 0.342420-460.568-6\\ 0.342420-512.178-6\\ 0.54268-512.178-6\\ 0.5428-512.178-5\\ 0.5428-512.178-5\\ 0.5428-512.178-5\\ 0.5428-528-5\\ 0.5428-528-5\\ 0.5428-528-5\\ 0.5428-528-5\\ 0.5428-528-5\\ 0.5488-5\\ 0.5488-5\\ 0.5488-5\\ 0.5488-5\\ 0.5488-5\\ 0.548$

2.1792 -0.0021767 4.6686 0.0011317 9.0446 0.014142 10.715 0.014142 11.372 0.014151 11.776 0.013682 12.140 0.013255 12.623 0.012660 13.642 0.012066 17.334 0.019402 21.485 0.029070 22.617 0.09556

0.028526

0.02798

0.025932 0.039743 0.042776 0.042103 0.041693 0.041383

0.056943

0.056312

0.055455

0.054854

.05480 .05476 .05473

0.054707 0.054682 0.054660

0.054660

0.054558 0.054552 0.054546

0.054536

0.054533

0.054534 0.054536 0.054539 0.054543 0.054548

0.054555 0.054562 0.054570

0.054579

0.054631

0.054644 0.054660 0.054678 0.054700 0.054729 0.054768

0.054822 0.054893 0.054973

0.054973 0.055044 0.055100 0.055144 0.055181 0.055216 0.055249 0.055285 0.055326

0.055520 0.055649 0.055877 0.056370

0.056193 0.04049

0.041491 0.041717 0.042057 0.042057 0.042658 0.041164 0.025874 0.026376

0.026887 0.027208 0.027460

0.027705

0.026274 0.012296

0.013826

21.485 22.617 23.105 23.434 23.756 24.227 25.449 30.893 33.872 24.577

34.577 34.908 35.144

35.391 35.811 37.368 44.422

45.970 46.354 46.535

46.535 46.653 46.750 46.842 46.928 47.000 47.054 47.096

.130 .158 .182

47.182 47.202 47.221 47.237 47.251 47.264 47.274 47.284 47.284

47.292 47.299 47.305

47.311 47.315 47.319

47.319 47.322 47.324 47.326 47.328

47.328 47.328 47.328 47.328 47.328 47.326 47.324 47.322 47.318

47.313 47.308 47.302

47.295

.281 .274 .266

.258 .248 .237 .224 .208 .187 .158

7.119 7.066 7.006

47.006 46.954 46.913 46.881 46.853 46.829 46.804 46.778 46.749 46.714 46.714 46.607

46.669 46.607 46.510 46.334 45.903 43.758 36.969 35.835

35.506 35.331 35.197

35.197 35.063 34.891 34.616 34.010 31.557 25.796

24.419

23.965 23.713 23.522

23.336

22.017 19.439 14.484 12.966

47

0.0

Vert Stress [kN/m²]

84.043E-6

347.12E-6 454.41E-6 611.62E-6 849.81E-6

0.0012317 0.0018845 0.0031018 0.0056541 0.012011 0.032739 0.14056

0.14056 1.2657 4.4414 4.9218 4.9799 4.9950 5.0046 5.0254 5.1567 7.2299 9.7972 0.0720

9.7972 9.9679 9.9909 9.9979 10.004 10.021

10.190 13.745 14.937 14.988 14.996 14.999 15.002 15.015 15.264 19.730 19.982 19.995 19.997

19.997 19.998 19.998 19.998 19.999 19.999 20.000 20.000

20.000 20.000 20.000 20.000 20.000 20.000 20.000 20.000 20.000 20.000 20.000

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19.998 19.998 19.998

19.998 19.997 19.997 19.994 19.976 19.452 15.142 15.011

15.002 15.000 14.999

14.999 14.998 14.996 14.989 14.950 14.135 10.260 10.026

10.006 10.001 9.9994

9.9994 9.9972 9.9931 9.9801 9.9074 8.8596 5.3894 5.0469

13.200 94.043E-6 13.200 99.465E-6 13.200 118.39E-6 13.200 142.90E-6 13.200 215.47E-6 13.200 215.47E-6 13.200 215.47E-6 13.200 347.12E-6 13.200 454.41E-6 13.200 454.41E-6 13.200 454.41E-6

Calc Level [mOD]

18.617

13.200 13.200 13.200 13.200 13.200 13.200 13.200 13.200 13.200 13.200 13.200 13.200 13.200 13.200

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Jasys

[m]

NR Railwav2

Mada	

by mdh

Date

08-May-2023

Checked

Printed 08-May-2023

Program Pdi C:\Users\mhi	•							
	541.41414	545.70707	13.20000	1.2148	13.200	4.9887	11.412	0
		543.93939	13.20000	1.2043	13.200	4.9958	11.675	0
			13.20000	1.1857	13.200	4.9995	11.893	0
	530.80808	540.40404	13.20000	1.1687	13.200	5.0033	12.120	0
	527.27273	538.63636	13.20000	1.1659	13.200	5.0118	12.423	0

PETER BRETT	Job No.	Sheet No.	Rev.
ASSOCIATES -READING	331201108		
le-le-Rice	Drg. Ref.		

Former H Ground Movement Assessment - Network Rail Assets

Jamblo	Airfield,	Hambl	
annue	All lielu,	пании	e-le-ric

asys

Estimated Ground Movements - Load Scenario 01
 Stresset

 Calc
 Vert
 Sum Princ

 Level
 Stress
 [RN/m²]

 13,200
 4,9668
 11,009

 13,200
 4,8517
 10.168

 13,200
 3,6361
 7,5869

 13,200
 0,015147
 3,2831

 13,200
 0,021572
 1.1444

 13,200
 0,0021560
 0.68385

 13,200
 0,0025056
 0.66835

 13,200
 0,0025056
 0.66835

 13,200
 0,0025056
 0.66835

 13,200
 0,0012562
 0.46423

 13,200
 0,0012562
 0.46423

 13,200
 0,0012562
 0.46423
 Location Y х Z[Level] z
 [m]
 [mOD]

 544.94949
 547.47475
 13.20000

 548.48485
 549.24242
 13.20000

 555.02020
 551.01010
 13.20000

 555.9556
 552.77778
 13.20000

 556.02027
 554.34545
 13.20000

 566.16162
 556.31313
 13.20000

 566.16162
 580.08041
 13.20000

 573.23222
 561.61616
 13.20000

 576.76768
 563.3834
 13.20000

 578.03834
 13.20000

 578.03834
 13.20000
 [mm] 1.1987 1.1018 0.72991 0.11297 -0.16700 -0.16700 -0.16745 -0.15503 -0.14196 -0.12965

Made by
mdh

Vert Strain [%]

Date 08-May-2023

Checked

	[m]	[m]	[mOD]	[mm]	Level [mOD]	Stress [kN/m ²]	[kN/m ²]	Strain [%]
	544.94949	547.47475	13.20000	1.1987	13.200	4.9668	11.009	0.014576
	552.02020	549.24242 551.01010	13.20000	1.1018 0.72991		4.8517 3.6361	10.168	0.015050 0.011325
	559.09091	552.77778 554.54545	13.20000	0.11297	13.200	0.51847	1.7171	-0.0014367 -0.0019256
	566.16162	556.31313 558.08081	13.20000	-0.16700	13.200 13.200	0.021572	0.85687	-0.0014569
	573.23232	559.84848 561.61616	13.20000	-0.16745	13.200	0.0043963 0.0025056	0.56780	-921.61E-6 -772.08E-6
	580.30303	563.38384 565.15152	13.20000 13.20000	-0.14196	13.200	0.0015622 0.0010394	0.42100	-661.90E-6 -577.39E-6
	583.83838 587.37374	566.91919 568.68687	13.20000 13.20000	-0.11853				-510.49E-6 -456.21E-6
	594.44444	570.45455 572.22222	13.20000	-0.099892	13.200	395.70E-6 304.28E-6	0.29839	-411.28E-6 -373.48E-6
	601.51515	573.98990 575.75758	13.20000	-0.085297 -0.079195	13.200	239.09E-6 190.73E-6	0.24719	-341.24E-6 -313.42E-6 -289.20E-6
		577.52525 579.29293	13.20000 13.20000	-0.073745	13.200	154.97E-6	0.20928	-289.20E-6 -267.91E-6
	612.12121 615.65657	581.06061 582.82828	13.20000 13.20000	-0.064456	13.200	105.87E-6 89.034E-6	0.18013	-249.08E-6 -232.30E-6
	619.19192 622.72727	584.59596 586.36364	13.20000 13.20000	-0.053581	13.200	75.623E-6 64.075E-6	0.15705	-217.27E-6 -203.74E-6
	626.26263 629.79798	588.13131 589.89899	13.20000 13.20000	-0.050579	13.200	55.432E-6 48.131E-6	0.13837	-191.49E-6 -180.37E-6 -170.23E-6
	633.33333 636.86869	591.66667 593.43434	13.20000	-0.045299 -0.042970 -0.040818	13.200	41.872E-6	0.12297	-170.23E-6 -160.95E-6
		595.20202 596.96970	13.20000 13.20000		13.200	32.187E-6 28.759E-6	0.11009	-152.43E-6 -144.58E-6
	647.47475	598.73737 600.50505	13.20000 13.20000					
	654.54545	602.27273	13.20000 13.20000	-0.036975 -0.035256 -0.033653 -0.032157	13.200	20.042E-6 18.254E-6	0.089838	-124.42E-6 -118.64E-6
	661.61616	605.80808 607.57576	13.20000 13.20000	-0.030757 -0.029446	13.200	16.615E-6	0.081767	-113.25E-6
	668.68687	609.34343 611.11111	13.20000 13.20000	-0.028216 -0.027061 -0.025973	13.200	13.411E-6 11.921E-6	0.074735	-103.52E-6 -99.123E-6
	675.75758	612.87879 614.64646	13.20000	-0.025973 -0.024949	13.200	11.101E-6	0.068568	-94.989E-6
	682.82828 686.36364	616.41414	13.20000					
	689.89899	619.94949	13.20000	-0.023069 -0.022206 -0.021389	13.200	7.7486E-6 7.2271E-6	0.058294	-80.766E-6
	696.96970	623.48485 625.25253	13.20000	-0.020614 -0.019880	13.200	6.8545E-6 6.0350E-6	0.053983	-74.796E-6 -72.051E-6
	704.04040	625.25253 627.02020 628.78788	13.20000 13.20000 13.20000	-0.019182	13.200	5.9605E-6	0.050121	-69.447E-6
	711.11111	630.55556 632.32323	13.20000	-0.018519 -0.017888 -0.017288	13.200	4.6939E-6 5.0664E-6	0.046646	-64.637E-6
	718.18182	634.09091 635.85859	13.20000	-0 016715	13 200	1 17038-6	0 043507	-60 2968-6
	725.25253	637.62626	13.20000 13.20000 13.20000	-0.016170 -0.015649 -0.015152	13.200	3.6508E-6	0.040660	-56.345E-6
	732.32323	641.16162 642.92929	13.20000	-0.014677 -0.014223	13.200	3.5018E-6	0.038073	-52.760E-6
	739.39394	644.69697 646.46465	13.20000	-0.013788	13.200	3.1292E-6	0.035713	-49.490E-6
	746.46465	648.23232 650.00000	13.20000	-0.013371 -0.012973 -0.012590	13.200	2.3097E-6	0.033554	-46.502E-6
X-Section	200.00000	600.00000 596.00000	18.80000	-0.051227	18.617	0.0	0.0	0.0
	204.00000	592.00000	18.80000 18.80000	-0.057432		0.0	0.0	0.0
	208.00000	584.00000 580.00000	18.80000	-0.064791 -0.068998	18.617	0.0	0.0	0.0
	212.00000	576.00000 572.00000	18.80000	-0 073619	18 617	0.0	0.0	0.0
	216.00000	568.00000 564.00000	18.80000 18.80000	-0.078714 -0.084352 -0.090617	18.617	0.0	0.0	0.0
	220.00000	560.00000 556.00000	18.80000 18.80000	-0.097608 -0.10545 -0.11428		0.0	0.0	0.0
	224.00000	552.00000 548.00000	18.80000	-0.11428	18.617	0.0	0.0	0.0
	228.00000	544.00000 540.00000	18.80000 18.80000	-0.13567	18.617	0.0	0.0	0.0
	232.00000	536.00000	18.80000	-0.16358	18.617		0.0 0.0 0.0	
		532.00000 528.00000 524.00000	18.80000 18.80000	-0.18058 -0.19963 -0.21993	18.617	0.0	0.0	0.0
	240.00000	520.00000 516.00000	18.80000 18.80000	-0.23820	18.617 18.617	0.0	0.0	0.0
	244.00000 246.00000	512.00000 508.00000	18.80000 18.80000	-0.19923 0.055764	18.617 18.617	0.0	0.0	0.0
	248.00000	504.00000 500.00000	18.80000	1.1491 1.4471	18.617 18.617	0.0	0.0	0.0
	252.00000	496.00000 492.00000 488.00000	18.80000	1.4959 1.4940 1.5099	18.617	0.0	0.0 0.0 0.0	0 0
		488.00000 484.00000	18.80000	1.5099 1.6631	18.617 18.617	0.0	0.0	0.0
	260.00000	480.00000 476.00000	18.80000	2.6709 3.0721	18.617	0.0	0.0	0.0
	266.00000		18.80000 18.80000	3.1276 3.1181	18.617 18.617	0.0	0.0	0.0
		464.00000 460.00000	18.80000	3.1136 3.1997	18.617 18.617	0.0	0.0	0.0
	272.00000 274.00000	456.00000 452.00000 448.00000	18.80000	3.9312	18.617 18.617	0.0	0.0	0.0
	278.00000	444.00000		4.6106 4.5992	18.617 18.617	0.0	0.0	0.0
	282.00000	440.00000 436.00000	18.80000	4.5815 4.6226	18.617	0.0	0.0	0.0
	286.00000	432.00000 428.00000	18.80000 18.80000	5.0296 5.9113	18.617 18.617	0.0	0.0	0.0
	290.00000	424.00000 420.00000	18.80000 18.80000			0.0		0.0
	292.00000	416.00000 412.00000 408.00000	18.80000 18.80000 18.80000	6.1129 6.9471	18.617	0.0	0.0	0.0
	298.00000	404.00000	18.80000	7.6341	18.617 18.617		0.0	0.0
	302.00000	400.00000 396.00000	18.80000	8.6295 8.8294	18.617 18.617	0.0	0.0	0.0
	304.00000	392.00000	18.80000	8.8468 8.8453	18.617	0.0	0.0	0.0
	310.00000	384.00000 380.00000	18.80000			0.0	0.0	0.0
	314.00000	376.00000 372.00000		10.166		0.0		0.0
	318.00000	368.00000 364.00000	18.80000	10.165	18.617	0.0	0.0	0.0
	322.00000	360.00000 356.00000	18.80000 18.80000	10.145 10.138	18.617 18.617	0.0	0.0	0.0
	326.00000	348.00000	18.80000	10.135 10.131 10.116	18.617	0.0		
	330.00000	344.00000 340.00000				0.0		0.0
	334.00000	336.00000 332.00000		8.8080		0.0		0.0
	338.00000	328.00000 324.00000 320.00000	18.80000 18.80000 18.80000	8.7198 8.7086 8.7168	18.617 18.617 18.617	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0
	342.00000	316.00000 312.00000	18.80000 18.80000 18.80000	8.7304	18.617	0.0	0.0	0.0
	346.00000	308.00000 304.00000	18.80000 18.80000 18.80000	8.7598 8.7650	18.617 18.617 18.617	0.0 0.0 0.0	0.0 0.0 0.0	0.0
	350.00000	300.00000	18.80000	8.7321 8.5150	18.617	0.0	0.0	0.0
	354.00000	292.00000 288.00000	18.80000	7.5159	18.617	0.0		
	358.00000 360.00000	284.00000 280.00000	18.80000 18.80000	7.3399 7.3525	18.617	0.0	0.0	0.0
	362.00000	276.00000	18.80000	7.3736	18.617	0.0	0.0	0.0

7 52 15	PETER BRETT	Job No.	
asys	ASSOCIATES -READING	3312	

Ground Movement Assessment - Network Rail Assets Estimated Ground Movements - Load Scenario 01

Job No.	Sheet No.	Rev.
331201108	3	
Drg. Ref.		
Made by mdh	Date 08-May-2023	Checked

Location Y Stresses : Sum Princ Vert Stress [kN/m²] Z[Level] Calc Level [mOD] 18.61 18.61 Vert Strain [%] [kN/m²] [m] [m] [mm] [mOD]
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18.617 18.617 0.0 0.0 0.0 0.0 0.0 0.0 J.C 0.0 0.0 1.0 18.61 0.0 18.617 18.617 18.617 18.617 18.617 18.617 18.617 18.617 18.617 0.0 0.0 0.0 0.0 0.0 $\begin{array}{c} 0.0 & 0.0 \\ 0.0 & 0.0 \\ 0.12955 & -179.40E-6 \\ 0.13738 & -190.24E-6 \\ 0.13738 & -202.06E-6 \\ 0.15530 & -2215.01E-6 \\ 0.15550 & -229.24E-6 \\ 0.15550 & -229.24E-6 \\ 0.16550 & -229.24E-6 \\ 0.21925 & -303.32E-6 \\ 0.23922 & -303.32E-6 \\ 0.23922 & -303.32E-6 \\ 0.25963 & -355.32E-6 \\ 0.33720 & -455.55E-6 \\ 0.33720 & -455.55E-6 \\ 0.41769 & -575.59E-6 \\ 0.41769 & -575.59E-6 \\ 0.41769 & -575.59E-6 \\ 0.53933 & -741.00E-6 \\ 0.53933 & -741.00E-6 \\ 0.53933 & -0.0012512 \\ 1.2357 & -0.0016241 \\ 0.73165 & -0.0016261 \\ 1.2357 & -0.0016261 \\ 1.2357 & -0.0016261 \\ 1.2357 & -0.0016261 \\ 1.2357 & -0.0016261 \\ 1.2357 & -0.0016261 \\ 1.2357 & -0.0016261 \\ 1.2357 & -0.001661 \\ 1.2557 & -0.001661 \\ 1.2557 & -0.00$ 0.0 -0.15243 -0.050116 -0.053022 -0.056178 -0.059613 -0.063364 -0.067471 -0.071981 -0.076952 -0.082451 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asys	PETER BRETT
	ASSOCIATES -READING

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 Stresses

 Vert
 Sum Princ

 Stress
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 14.990
 34.046

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 29.088

 10.085
 25.106

 10.011
 24.195

 10.011
 24.795

23.785 23.459 23.061

0.001

9.9854 9.9246 8.7829 5.2597 5.0292 5.0292 5.0051 4.9961 4.9821

4.9821 4.9199 4.2087 0.65159 0.071919 0.018323

0.0071061 0.0034689 0.0019532 0.0012115 804.89E-6

Vert Strain

0.039896

0.040561 0.040946 0.041292 0.041741 0.042504

0265

Job No.	Sheet No.	Rev.
331201108		
Dra Ref		

Date 08-May-2023

Checked

Former Hamble Airfield, Hamble-le-Rice

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Location Y

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Ground Movement Assessment - Network Rail Assets Estimated Ground Movements - Load Scenario 01

Z[Level]

[mOD]

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2.4145 2.4151 2.4362 2.4442 2.3813 2.0028 1.3135 1.1476 1.1281 1.1407 1.1454 1.0915

1.1454 1.0915 0.80237 0.10310 -0.15962 -0.21262 -0.21247 -0.19788 -0.18037 -0.16354

-0.16354

Time 14:43

Printed 08-May-2023

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